Calculations AECC Southwest London SFRA Job no. Job Title Level 2 Site Assessments Document No. Revision Originator Checked **Approved** Date 11/11/16 JAB JJI CP Introduction These calculations have been undertaken to estimate the existing peak runoff rates from the development sites considered within the Level 2 Strategic Flood Risk Assessment. Target peak runoff rates have then been provided based on national and regional planning policy requirements and best practice guidance. The calculations presented herein have been undertaken in accordance with the methods and recommendations set out in Chapter 24 of the SuDS Manual (CIRIA C753, 2015). There are several methods that can be used for estimating peak runoff rates, these can Selection of Runoff broadly be separated into the following two categories: **Estimation** a) Methods for estimating runoff from greenfield sites; and, Method b) Methods for estimating runoff from previously developed sites. Table 24.1 of the SuDS Manual provides a summary of the methods available and details of when they should be utilised. For these calculations, the Institute of Hydrology 124 (IH124) runoff estimation method (Marshall and Bayliss, 1994) has been used incorporating recommendations within Section 24.5 of the SuDS Manual for using this method for previously developed sites. IH124 The flood frequency (flows for different return period events) estimation procedure consists of Methodology three stages: 1) Estimate the Q_{BAR} (mean annual flood); 2) Select an appropriate growth curve; and, 3) Evaluate the full flood frequency curve. 1) Estimate the Q_{BAR} (mean annual flood): The IH124 equation for Q_BAR is given in Equation 24.3 of the SuDS Manual (CIRIA C753, 2015). IH124: Catchment descriptor equation 24.3 $Q_{BAR(rural)} = 0.00108 \ AREA^{0.89} \times SAAR^{1.17} \times SOIL^{2.17}$ where: = mean annual flood (a return period in the region of 2.3 years) AREA = area of the catchment in km2 SAAR = Standard Average Annual Rainfall for the period 1941-1970 in mm SOIL = soil index, which is a value found from the FSR soil maps or the WRAP map of the Wallingford procedure, and represents an estimate of the proportion of runoff from the catchment surface* Where the site is less than 50 ha, the formula should be applied for 50 ha and the result factored based on the ratio of the actual site area and the applied area (50 ha). The input parameters for Equation 24.3 have been attained from the following sources / methods: AREA - All of the sites assessed are less than 50 hectares in size therefore a value of 0.5km² (50ha) has been used for AREA and the Q_{BAR} value has been linearly factored for the site area. Cont.

- SAAR Standard Average Annual Rainfall This value for each of the development sites has been attained from the maps within the MicroDrainage Source Control software.
- SOIL In accordance with the recommendations in Section 24.5 of the SuDS Manual; for previously developed sites the SOIL value for Flood Studies Report (FSR) Soil Type 5 has been used. Soil Type 5 refers to soil with a very low permeability such as solid rock and is therefore an appropriate representation of previously developed land. The value used within these calculations from the FSR (NERC, 1975) is 0.53.

2) Select an appropriate growth curve

QBAR can be factored by the UK FSR growth curves (NERC, 1997) for return periods less than 2 years and NERC (1993) for all other return periods to obtain peak flow estimates for required return periods.

These regional growth curves are constant throughout a region and are given in Table 24.2 of the SuDS Manual.

UK and Ireland growth curve factors (after NERC, 1993)									
Hydrometric area	Return period								
	11	2	5	10	25	30 ²	50	100	500
1	0.85	0.90	1.20	1.45	1.81	1.99	2.12	2.48	3.25
2	0.87	0.91	1.11	1.42	1.81	1.99	2.17	2.63	3.45
3	0.86	0.94	1.25	1.45	1.70	1.75	1.90	2.08	2.73
9	0.88	0.93	1.21	1.42	1.71	1.80	1.94	2.18	2.86
10	0.87	0.93	1.19	1.38	1.64	1.70	1.85	2.08	2.73
4	0.83	0.89	1.23	1.49	1.87	1.99	2.20	2.57	3.62
5	0.87	0.89	1.29	1.65	2.25	2.55	2.83	3.56	5.02
6/7	0.85	0.88	1.28	1.62	2.14	2.40	2.62	3.19	4.49
8	0.78	0.88	1.23	1.49	1.84	1.98	2.12	2.42	3.41
Ireland	0.832	0.95	1 20	1 37	1 60	1.65	1 77	1.96	2 40

3) Evaluate the full flood frequency curve

The peak flow rates for each return period can then be estimated as the product of Q_{BAR} and the relevant growth factor.

The SuDS Manual and the Non-Statutory Technical Standards for SuDS (Defra, 2015) recommend that the rate of runoff does not increase following development for the 1 in 1 year and 1 in 100 year events. The existing peak runoff rates from the development sites have been estimated for QBAR, Q(1), Q(30) and Q(100).

Target Post Development Runoff Rates

Greenfield Runoff Rates:

In accordance with the SuDS Manual, the peak greenfield runoff rate for each development site has been estimated as 2 litres per second per hectare.

London Plan:

The London Plan has a policy requirement that all development "aims to achieve greenfield runoff rates" to ensure that runoff from development sites is reduced as much as possible. In
meeting this policy requirement, the Sustainable design and Construction Supporting Planning
Guidance (2014) document provides further detail and guidance on managing runoff. It states
that there is a minimum expectation to deliver 50% attenuation of a site's (prior to redevelopment) surface water runoff at peak times. It also states that "on previously developed
sites, runoff rates should not be more than three times the calculated greenfield rate".

Minimum Realistic Design Discharge Rate

Where target post development runoff rates have been calculated to be less than 5 litres per second, a value of 5 litres per second has been specified as it is widely considered unrealistic to design drainage systems which discharge at rates lower than this without risk of frequent blockage.

Appendix B